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Numerical analysis of the transient pressure damping in a single polymeric pipe with a leak.

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ABSTRACT

In the last decades transient test-based techniques (TTBTs) are imposing for fault detection in transmission mains. Within TTBTs, the direct transient analysis (DTA) allows identifying the defects directly in the pressure signal. A possible DTA procedure is based on the analysis of the damping of the pressure peaks. In this paper, it is shown that the pressure decay in a polymeric leaky pipe depends exponentially on leak size and location and the pressure at the leak. It is also pointed out that, for a given transient, the same damping of the pressure peaks may result from different pipe systems (e.g., with the leak of a different size in a different location). Such a result merits further insights also by means of experimental tests in different pipe systems.

KEYWORDS

leak detection, pressure damping, transients, transmission mains

1. Introduction

It is more than a century ago that, by the evidence of clear laboratory experiments, Joukowsky (1900) pointed out that in a pressurized pipe transient pressure waves are partially reflected by any change in the physical structure of the pipe. Possible changes are defects like, as an example, a leak, a partial blockage due to the deposition of sediment, and a deterioration of the pipe wall. A change may be also due to a device (e.g., a partially closed in-line valve) or a branch. As a consequence of such a property of the pressure waves, in principle any change in the pipe can be detected by means of transient tests. However, the fault detection procedure may fail if the way how the tests are executed and the methods used for analyzing the results are not appropriate. Notwithstanding the clear evidence of the principle on which they are based, till early nineties of the last century, transient test-based techniques (TTBTs) disappeared into an almost full oblivion surrounded by skepticism. Even in the solitary paper by Babbitt, Amsbary, and Gwinn (1920), after having highlighted the potential of the TTBTs, the Authors clearly show their preference for steady-state tests to detect a leak in a pipe. The true reason of such an attitude of both the Academia and technicians is the unjustified assumption that large — and then potentially dangerous for the pipes — pressure waves must be generated for a reliable fault detection. Only recently, such a

19 prejudice has been rebutted on the basis of convincing results of both laboratory and
20 field tests. Specifically, it has been shown that small amplitude pressure waves allow
21 a reliable fault detection (e.g. Meniconi et al. 2011b, 2017).

22 This said, it is evident that, particularly for transmission mains (TMs), for a number
23 of reasons competitiveness and favorable prospects of TTBTs are undeniable with re-
24 spect to other methods. As an example, the short duration of the transient tests and
25 the fact that only pressure measurements are needed are certainly in favor of TTBTs
26 with respect to steady-state tests. TTBTs are even more attractive since they allow
27 detecting devious defects (e.g., partial blockages) which do not give rise to any exterior
28 sign (Duan et al. 2011, 2013; Lee et al. 2008; Louati and Ghidaoui 2017; Louati et al.
29 2017; Zhang et al. 2018). The alternative option, i.e. the in-line type technologies with
30 tethered and free-swimming sensors inserted into the pipelines, is more demanding
31 from both the economic and logistic point of view. In fact such technologies imply
32 the construction of quite expensive as well as stable access points for the insertion
33 and extraction of the sensors. Furthermore, such a need prevents the utilization of
34 the in-line type technologies for executing an extemporaneous check of the pipe systems
35 which, on the contrary, is possible when TTBTs are used (e.g. Meniconi et al. 2011a).

36 In the view of the above, with the aim of improving the performance of TTBTs, now-
37 days attention is focused on refining procedures for executing reliable transient tests
38 and appropriate methods for analyzing the experimental data. With regard to the lat-
39 ter point, in literature several approaches have been proposed to maximize the amount
40 of information which can be extracted from data collected during transient tests. A
41 basic distinction concerns whether or not a numerical model simulating the transient
42 tests is used. If it is, we can speak in terms of Inverse Transient Analysis (ITA) where
43 the characteristics of the defect (e.g., type, location, and severity) are the unknowns
44 of the problem and are obtained within a calibration procedure by minimizing the
45 difference between the measured data and the numerical model results. The success of
46 such a method, proposed by Liggett and Chen (1994), depends strongly on the degree
47 of knowledge of the pipe system feature (e.g., boundary and initial conditions, and
48 geometrical and mechanical characteristics of the pipes). To fulfill such a requirement,
49 the preliminary measurement at several sections of both pressure and discharge and a
50 detailed inspection of the pipe components is needed. Moreover, within ITA a proper
51 balance between using the most appropriate governing equations and minimizing the
52 number of the parameters to be estimated is not an easy task. As an example, the
53 role of the unsteady friction (UF) and viscoelasticity (VE), and then if it is the case of
54 including or not such a term in the momentum and continuity equation respectively,
55 is not known a priori (e.g. Duan et al. 2010a; Nixon and Ghidaoui 2007). In general,
56 it can be said that the more complete the model — and then the better its perfor-
57 mance — the more complex the preliminary phase in which it must be assessed before
58 the unknowns of the problems (i.e., the characteristics of the defects) are obtained.

59 The criticality of such constraints are merely suggested when numerical or laboratory
60 experiments are examined but they can exceed the most pessimistic forecast when
61 dealing with real TMs. As an example, in Meniconi, Brunone, and Frisinghelli (2018),
62 field and numerical experiments show the unexpected remarkable effect of the short
63 minor branches, even if inactive, and small defects (e.g., the malfunction of a valve
64 which allows the flow of a small discharge) on the transient response of a TM. More-
65 over, the evaluation of the actual pressure wave speed — a key parameter within
66 TTBTs — may experience a serious difficulty (e.g. Meniconi et al. 2015).

67 To the end of surpassing the intrinsic complexity of ITA, instead of simulating accu-
68 rately the full experimental pressure traces, a Direct Transient Analysis (DTA) can be

69 performed in which the effects of the defects are identified directly in the pressure sig-
70 nal. Within DTA, two possible approaches can be followed (Colombo, Lee, and Karney
71 2009): the time-domain reflectometry (e.g. Jönsson 1970; Jönsson and Larson 1992;
72 Brunone 1999), and the transient damping method (Wang et al. 2002; Nixon, Ghi-
73 daoui, and Kolyshkin 2006). In the first case, the characteristics of the pressure waves
74 reflected by the defect/device are considered whereas in the second case the examined
75 feature is the damping of the Fourier components for each mode of the pressure signal.
76 The relative importance of the damping and reflective effects from the leak has been
77 examined in Duan et al. (2010b).

78 In literature, the properties of the reflected pressure waves have been quite extensively
79 explored. In fact, laboratory and field tests, as well as numerical experiments both in
80 the time and frequency domain (e.g. Brunone 1999; Covas and Ramos 2010; Lee et al.
81 2006, 2005; Mpesha, Gassman, and Chaudhry 2001; Mpesha, Chaudhry, and Gassman
82 2002), have pointed out the main factors influencing the reflected pressure waves. On
83 the contrary, since the identification of the mechanisms governing the damping of
84 the pressure peaks in a pipe with a leak deserved less attention (Wang et al. 2002;
85 Nixon, Ghidaoui, and Kolyshkin 2006), particularly for polymeric pipes, the aim of
86 this paper is to explore in more details the behavior of the pressure peak damping in
87 a high-density polyethylene pipe (HDPE).

88 As an important premise, it must be noted that after fast closure maneuvers (as those
89 usually executed for fault detection), in an integer single pipe the damping of the pres-
90 sure peaks — hereafter referred to as intrinsic damping — is mainly due to UF and VE
91 since in such transients the relevance of the steady-state friction is quite negligible. As
92 highlighted by Duan et al. (2010a), UF plays a significant role in elastic pipes whereas
93 VE is noticeable in polymeric ones. In elastic pipes, the presence of a leak changes
94 significantly the transient response of the system (Wiggert 1968) with the importance
95 of UF depending on the percentage of the flow through the leak with respect to the
96 mean flow (Nixon and Ghidaoui 2007). In terms of the pressure signal, the effect of
97 the leak is an additional damping of the pressure peaks with respect to the intrinsic
98 one (e.g. Wang et al. 2002; Colombo, Lee, and Karney 2009). Pressure signals of Fig. 1
99 indicate that the same applies to HDPE pipes with an additional pressure peak decay
100 quite larger than the intrinsic one (i.e., the damping, due mainly to VE, happening
101 in the integer pipe), according to Duan et al. (2012); in the figure, H = piezometric
102 head (with the subscripts M and 0 indicating the downstream end section of the pipe
103 and the initial steady-state conditions, respectively), $C_\ell A_\ell$ is the leak effective area,
104 C_ℓ and A_ℓ are the leak discharge coefficient and area, respectively, and the subscript
105 ℓ refers quantities to the leak. Precisely these plots show that the larger the leak, the
106 larger the damping of the pressure peaks with respect to the integer pipe. In tests of
107 Fig. 1 — executed at the Water Engineering Laboratory (WEL) of the University of
108 Perugia, Italy — a leak of a different size has been considered at a distance $s_\ell = 60.84$
109 m from the supply reservoir in a single HDPE pipe (Fig. 2) with a length $L = 166.28$
110 m, an internal diameter $D = 93.3$ mm and pressure wave speed $a = 377.15$ m/s. In
111 all these transients, the same value of the discharge downstream of the leak, $Q_{0,d}$ (=
112 4.24 L/s), has been assumed as an initial condition (the subscript d refers to the pipe
113 downstream of the leak). As a consequence, the same Allievi-Joukowski overpressure,
114 $\Delta H_{AJ} = aV_{0,d}/g$ (= 24 m) has been generated, with V = mean flow velocity, and g
115 = gravitational acceleration.

116 Having in mind the proposed use of the damping of the pressure peaks as a possible
117 feature for leak detection by means of unsteady-state tests, in this paper attention is
118 focused on the effect of leak size and location for a given pipe in transient conditions.

119 The behavior of the total (i.e., the sum of the intrinsic and leak-induced damping)
120 pressure peak damping is examined by means of numerical experiments executed by
121 using a 1-D model calibrated on the basis of a huge series of tests carried out at WEL
122 on polymeric pipe systems with different characteristics — e.g., with a leak (Ferrante
123 et al. 2014; Capponi et al. 2017), both an extended (Meniconi, Brunone, and Ferrante
124 2012) and discrete (Meniconi et al. 2016) partial blockage, branch (Meniconi et al.
125 2011c), and a partially closed in-live valve (Meniconi et al. 2012). Therefore this paper
126 differentiates significantly from the literature where, within the hypothesis of small
127 amplitude transients, attention has been focused on i) the analytical solutions for leak
128 detection and sizing — corroborated by laboratory tests on a small diameter copper
129 pipe — of the linearized governing equations (Wang et al. 2002), and ii) the range of
130 validity of such a method with an assessment of the effect of transient amplitude and
131 noticeable comments about its applicability to non simple systems and nonuniqueness
132 of the solution (Nixon, Ghidaoui, and Kolyshkin 2006).

133 2. Materials and Methods

134 As anticipated above, attention is focused on the total damping of the pressure peaks
135 which happens in a single-diameter polymeric pipe with a leak placed at a distance
136 s_ℓ from a constant head supply reservoir (Fig. 2), hereafter referred to as *leaky pipe*.
137 Transients are generated by the complete closure of the valve installed at the down-
138 stream end section of the pipe. For the sake of clarity, firstly transients caused by an
139 instantaneous closure are examined and successively the role of the duration of the
140 maneuver is discussed.

141 According to literature (e.g. Ramos et al. 2004; Meniconi et al. 2014), as a prelimi-
142 nary step, the time-history of the pressure maxima at the downstream end section of
143 the pipe (section M in Fig. 2), $H_{M,max}$, is assumed as the representative feature of the
144 considered transients; thereafter the whole transient pressure trace will be taken into
145 account.

146 In dimensionless terms, the pressure local maxima, $h_{M,max}^*$, at the downstream end
147 section of the pipe are defined as:

$$h_{M,max}^*(t^*) = \frac{H_{M,max}(t^*) - H_{F,d}}{\Delta H_{AJ}}, \quad (1)$$

148 where the subscript F indicates the final steady-state condition and the dimension-
149 less time, t^* , is equal to t/τ , with t = time elapsed since the beginning of the transient,
150 and $\tau = 2L/a$ being the characteristic time of the pipe. The available 1-D model —
151 calibrated, as mentioned, by means of transient tests executed on HDPE pipe systems
152 — has then been used to identify quantities affecting the total pressure peak damp-
153 ing i.e., the time-history of $h_{M,max}^*$. In the executed analysis, $V_{0,d}$, $H_{0,\ell}$, $C_\ell A_\ell$, and s_ℓ
154 have been assumed as possible characteristic quantities for the given pipe material (i.e.,
155 high-density polyethylene). In fact, on one side, for the considered maneuver and given
156 pressure wave speed, $V_{0,d}$ is responsible for the value of ΔH_{AJ} which is the pressure
157 wave that, injected into the system, successively will damp because of the presence
158 of the leak, the other boundary conditions, as well as pipe material behavior. On the
159 other side, according to literature (Liou and Tian 1995; Liou 1998; Ferrante et al.
160 2014), for the given pipe characteristics, $H_{0,\ell}$ and $C_\ell A_\ell$ characterize fully the behavior

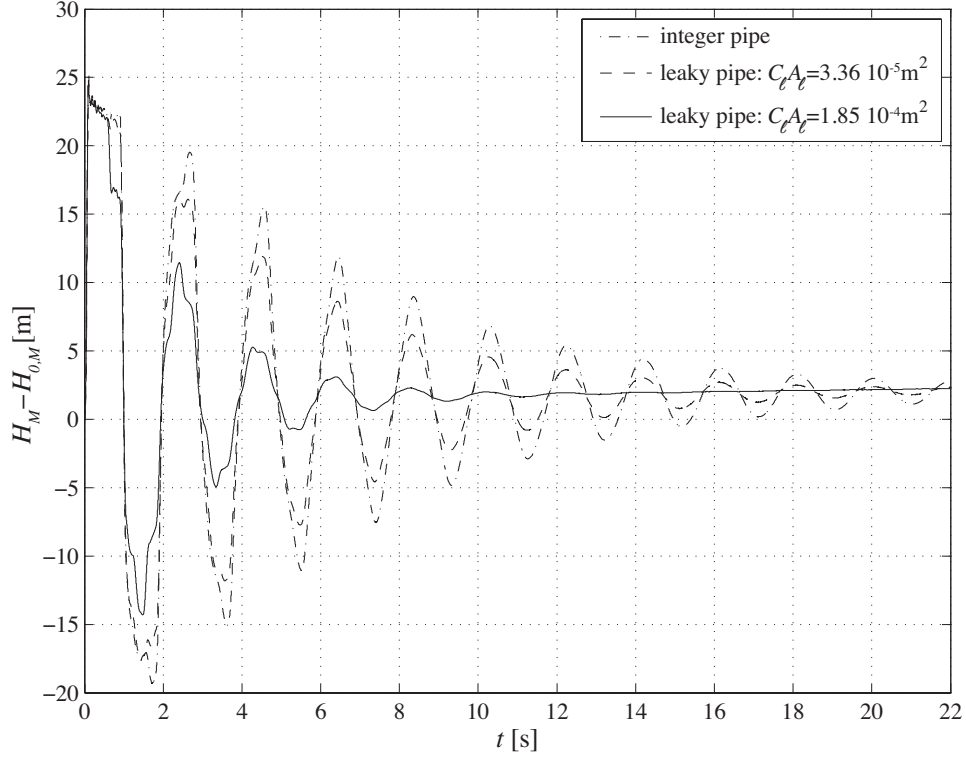


Figure 1.: Experimental pressure signal in an integer pipe vs. those in the same pipe with a leak of different size.

161 of the leak whereas s_ℓ plays a crucial role in the mechanisms of interaction between
 162 the pressure waves and the boundary conditions and then on the pressure damping. In
 163 the below numerical experiments, the mentioned laboratory pipe has been considered
 164 as reference. Accordingly, in dimensionless terms, the following relationship can then
 165 be written:

$$h_{M,max}^* = h_{M,max}^* [N_{0,d}, h_{0,\ell}^*, \Sigma^*, \delta] (t^*), \quad (2)$$

166 where:

$$N_{0,d} = V_{0,d} D / \nu \quad (3)$$

$$\delta = 1 - s_\ell / L \quad (4)$$

$$\Sigma^* = A / (C_\ell A_\ell) \quad (5)$$

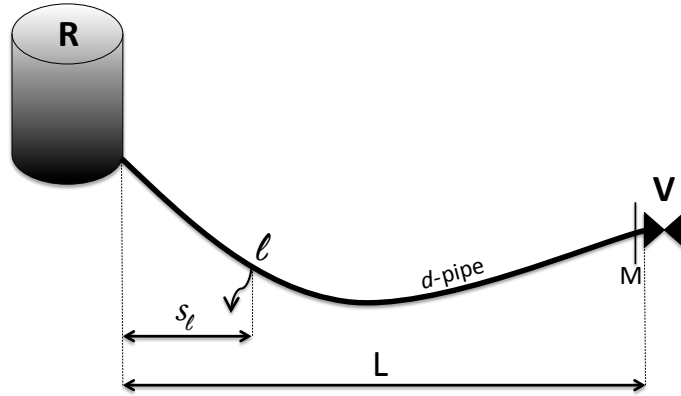


Figure 2.: Sketch of the single-diameter pipe with a leak (R = supply reservoir, L = pipe length, ℓ = leak; s_ℓ = distance between the supply reservoir and the leak; d -pipe = pipe downstream of the leak; M = downstream end section; and V = maneuver valve.)

$$h_{0,\ell}^* = \sqrt{2gH_{0,\ell}/a} \quad (6)$$

167 with $N_{0,d}$ = initial Reynolds number in the d -pipe, A = pipe area, and ν =
 168 kinematic viscosity.

169

170 3. The pressure peak damping behavior

171 According to Covas et al. (2004, 2005) — who examined the case of the integer single
 172 pipe — and Wang, Lambert, and Simpson (2005) and Meniconi et al. (2014) — for
 173 the case of a pipe with a partially closed in-line valve — a possible formulation of the
 174 function of Eq. (2) is in terms of an exponential law:

$$h_{M,max}^*(t^*) = \alpha e^{-\beta t^*}, \quad (7)$$

175 where coefficient α takes into account the initial conditions, whereas β , the decay coef-
 176 ficient, reflects both the intrinsic and leak-induced damping (i.e., the total damping).
 177 Such an assumption — anticipated in Brunone et al. (2015) — is based on experimen-
 178 tal evidence. In fact, for the considered leaky pipe (i.e., for given pipe material, L , D ,

179 and a), the behavior of the total pressure damping is clearly of an exponential type
 180 (Fig. 3).

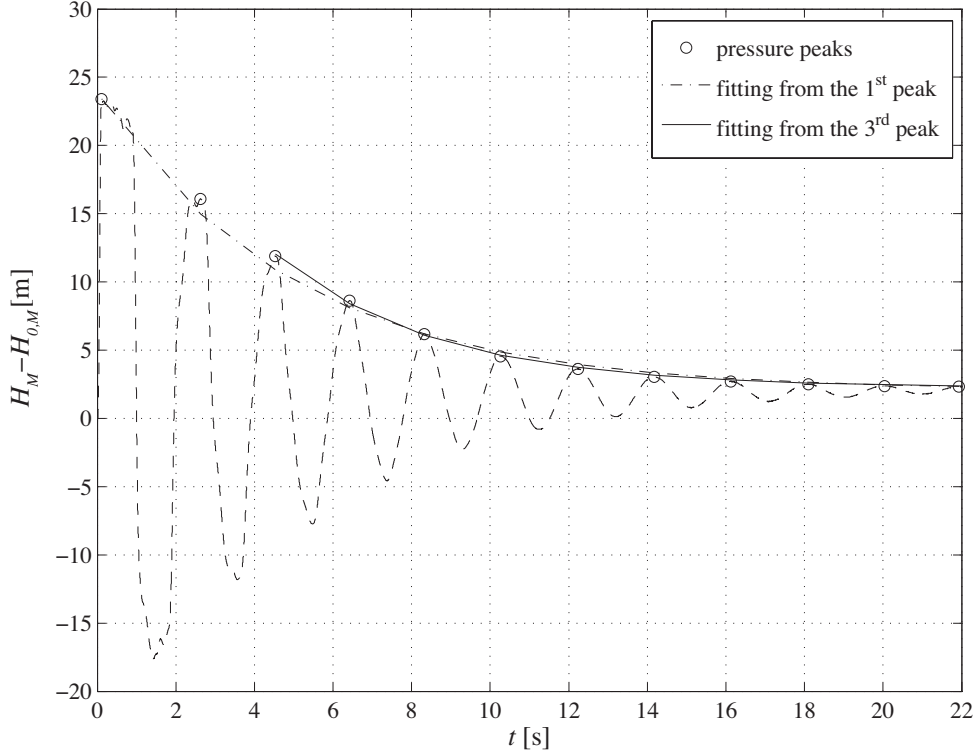


Figure 3.: Fitting of the pressure peaks in a leaky laboratory pipe ($C_\ell A_\ell = 3.36 \cdot 10^{-5} \text{ m}^2$).

181 Within the numerical experiments, by changing the value of the dimensionless pa-
 182 rameters $N_{0,d}$, $h_{0,\ell}^*$, Σ^* , and δ , numerical pressure signals have been obtained and for
 183 each of them the coefficients α and β have been evaluated. As highlighted in Fig. 3, the
 184 quality of the fitting improves if the starting time of the analysis does not include the
 185 first characteristic time (Wang et al. 2002). Precisely, the coefficient of determination,
 186 R^2 , is equal to 0.9945 and 0.9983 for the fitting from the first and the third peak,
 187 respectively. This is due to the fact that in the first phases of the transient, the shape
 188 of the pressure signal is strongly influenced by the single reflected pressure waves, with
 189 a negative effect in terms of periodicity of the pressure signal.
 190 In Fig. 4 coefficients α and β are reported as a function of the dimensionless param-
 191 eters (3)-(6). These plots of Fig. 4 point out the relevance of $h_{0,\ell}^*$, Σ^* , and δ as well as
 192 the much smaller importance of $N_{0,d}$. With regard to the last dependence, the slight
 193 decrease of α with $N_{0,d}$ is a direct consequence of Eq. (1), whereas the almost constance
 194 of β implies that the entity of the injected pressure wave does not affect significantly
 195 the damping of the pressure peaks. The clear dependence of both α and β on $h_{0,\ell}^*$ and
 196 Σ^* confirms the experimental results reported in Fig. 1 and literature (e.g. Liou and
 197 Tian 1995; Liou 1998). Precisely, the larger $h_{0,\ell}^*$, and then the larger the pressure at
 198 the leak, $H_{0,\ell}$, the smaller the leak-induced damping. Moreover, the larger Σ^* , and
 199 then the smaller the leak size, $C_\ell A_\ell$, the smaller its effect during the transient. Less
 200 straightforward is the role of the leak location on β . According to Fig. 4, the smaller
 201 δ , i.e. the closer the leak to the end valve (i.e., the larger s_ℓ) where the pressure wave

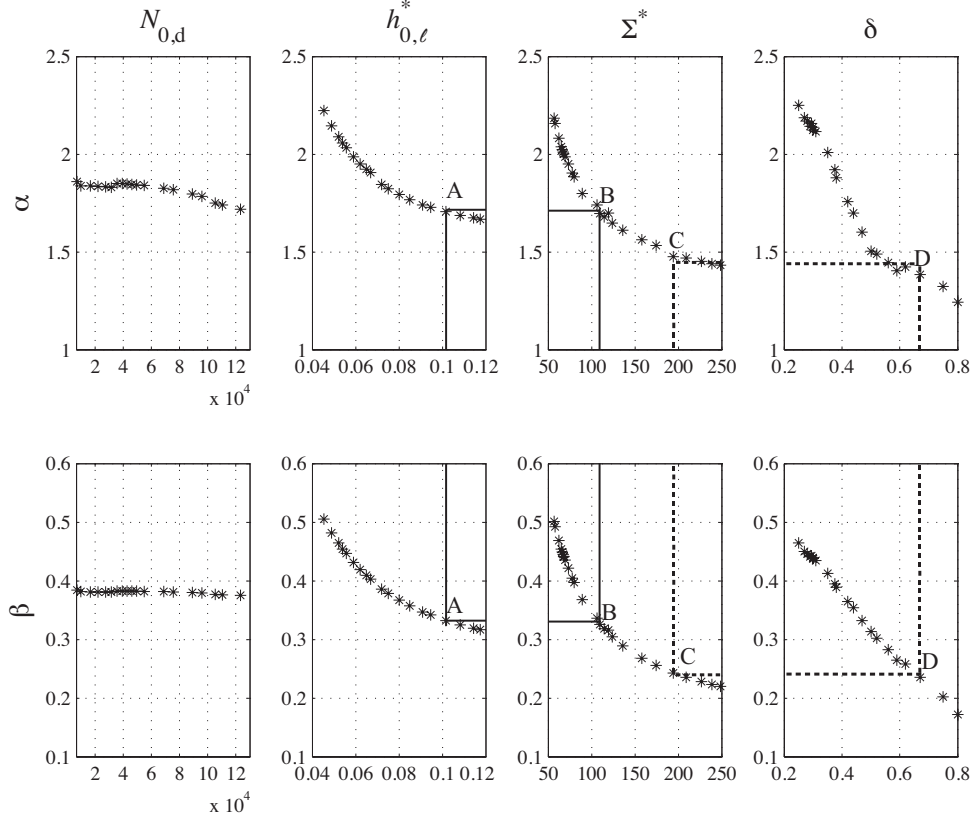


Figure 4.: Initial value coefficient, α , and decay coefficient, β , of Eq. (7), as functions of the dimensionless parameters $N_{0,d}$, $h_{0,\ell}^*$, Σ^* , and δ with selection of cases with the same value of α and β (couples A and B, and C and D).

202 is generated, the larger β as shown in numerical pressure signals of Fig. 5 where a
 203 given leak ($C_\ell A_\ell = 8.0 \cdot 10^{-5} \text{ m}^2$) is placed at two different locations (δ equal to 0.30
 204 and 0.85, respectively). In such a behavior, which merits an experimental check, the
 205 key role is played by the frequency of the reflections at the leak.

206 The analysis of Fig. 4 plots suggests that the same value of the coefficients α and
 207 β can characterize the damping of the pressure peaks of transients in different pipe
 208 systems (i.e., with a leak of a different size in a different location). In Fig. 4, two
 209 possible cases are highlighted (couple A and B, and couple C and D, respectively).

210 The first couple of pressure signals with the same coefficients α ($= 1.70$) and β ($=$
 211 0.33), labelled as A and B respectively, concerns two quite different leaky pipes. Specif-
 212 ically, for given $N_{0,d}$ ($= 4.1 \cdot 10^4$) and δ ($= 0.4$), i.e., for the same $V_{0,d}$ ($= 0.44 \text{ m/s}$) —
 213 and then ΔH_{AJ} ($= 16.92 \text{ m}$) — and leak location, s_ℓ ($= 99.77 \text{ m}$), in case A, $\Sigma^* =$
 214 85.46 ($C_\ell A_\ell = 8.0 \cdot 10^{-5} \text{ m}^2$) and $h_{0,\ell}^* = 0.102$ ($H_{0,\ell} = 75 \text{ m}$) whereas in case B, it is Σ^*
 215 $= 106.25$ ($C_\ell A_\ell = 6.4 \cdot 10^{-5} \text{ m}^2$) and $h_{0,\ell}^* = 0.074$ ($H_{0,\ell} = 40 \text{ m}$). This means that two
 216 leaks with a different size and initial pressure cause the same pressure peak damping.
 217 The differences between cases C and D ($\alpha = 1.44$ and $\beta = 0.24$) are leak location (δ
 218 equal to 0.4 and 0.67, respectively) and size ($C_\ell A_\ell$ equal to $5.0 \cdot 10^{-5} \text{ m}^2$ and $8.0 \cdot 10^{-5}$
 219 m^2 , respectively). In other words, even if the leak location and size change significantly
 220 — the difference in the location is about the 27% of the total length — the pressure

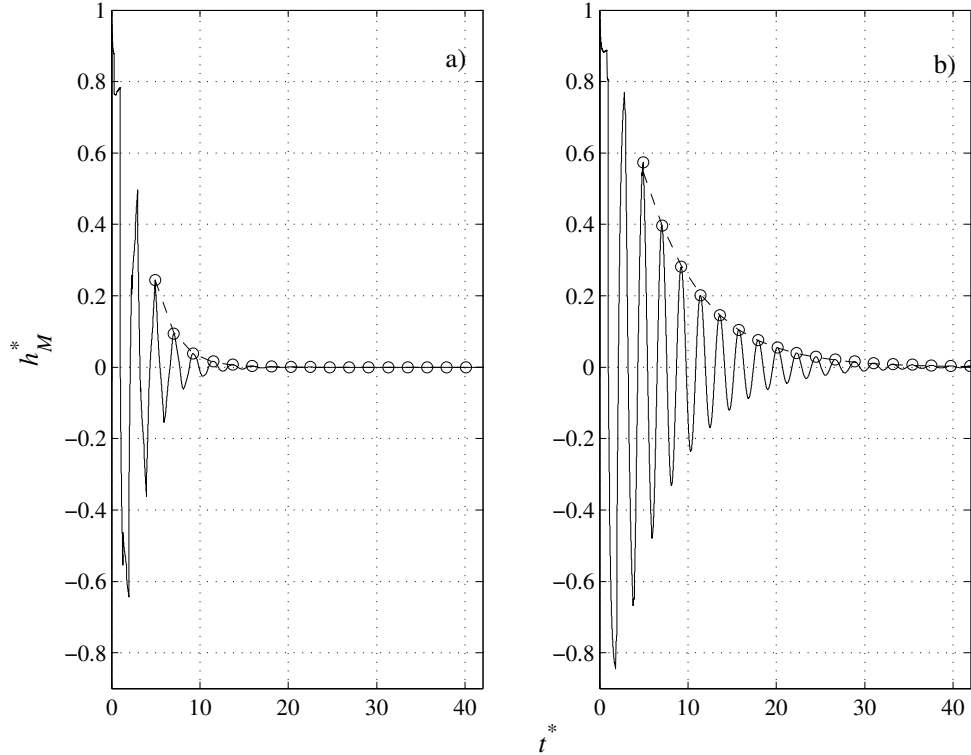


Figure 5.: Numerical pressure signals in a pipe with a given leak ($C_\ell A_\ell = 8.0 \cdot 10^{-5} \text{ m}^2$) at a different location: a) $\delta = 0.30$ ($s_\ell = 116.40 \text{ m}$), b) $\delta = 0.85$ ($s_\ell = 24.94 \text{ m}$).

221 damping at the end section of the pipe is the same. It is worthy of noting that the
 222 damping of the pressure peaks is not the only common feature between cases A and
 223 B, and C and D, respectively. In fact, as shown in Figs. 6 and 7, the whole pressure
 224 signals, both in dimensionless (h_M^*) and dimensional (H_M) terms, are almost indistin-
 225 guishable with the exception of the first characteristic time. In fact, as shown in Fig.
 226 8, for the pressure signals of Figs. 6b and 7b, the leak effect (i.e., a pressure drop) is
 227 evident in the first phases of the pressure signals.

228 Having in mind that in real pipe systems it may be quite difficult to execute fast ma-
 229 neuvers — unless a proper device is used (e.g. Brunone, Ferrante, and Meniconi 2008;
 230 Taghvaei, Beck, and Boxall 2010) or a small-diameter side valve is installed (Stephens
 231 et al. 2011) — the effect of the duration of the closing maneuver, T , on the transient
 232 response has been explored. Moreover the interest for slower maneuvers derives from
 233 the fact that for complex systems the damping of the pressure peaks is easier to evalu-
 234 ate with respect to single pressure waves reflected in the first characteristic time as
 235 within the time domain reflectometry. Specifically, for cases C and D, as an example,
 236 different values of Θ ($= T/\tau$) have been considered ($\Theta = 0.5; 1; 5; \text{ and } 10$). As clearly
 237 shown in Fig. 9, the pressure signals for cases C and D are almost indistinguishable
 238 for all the considered values of Θ .

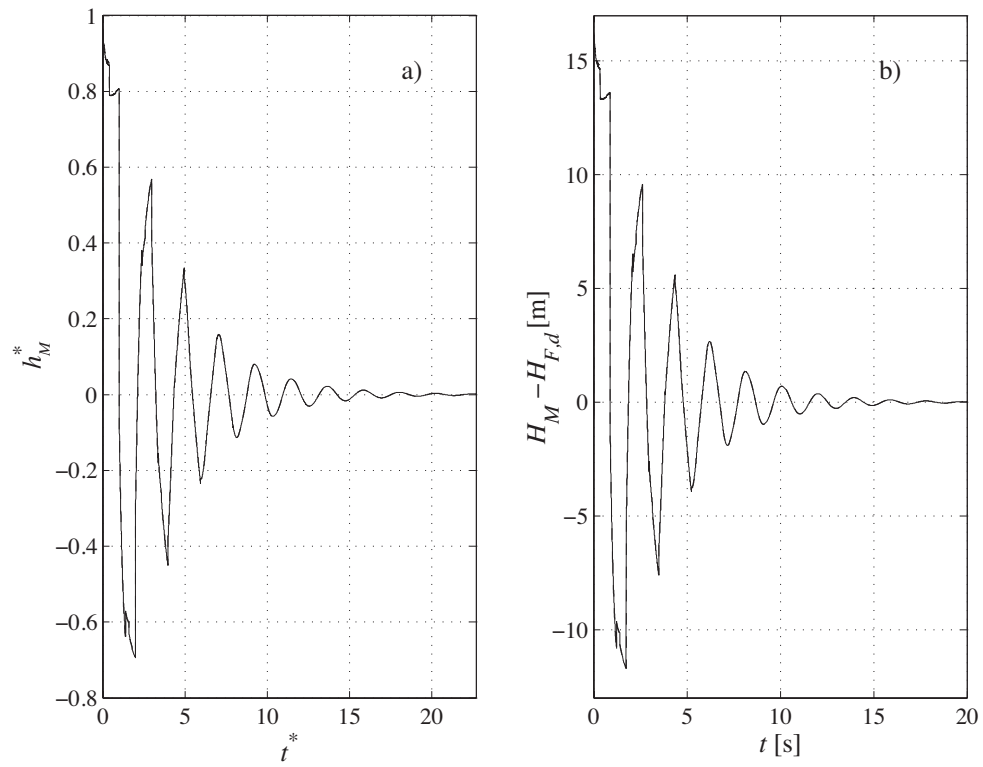


Figure 6.: Transients with the same value of the coefficients α and β of Eq. (7) for couple A (solid lines) and B (dashed lines) of Fig. 4: a) dimensionless pressure signals; b) dimensional pressure signals.

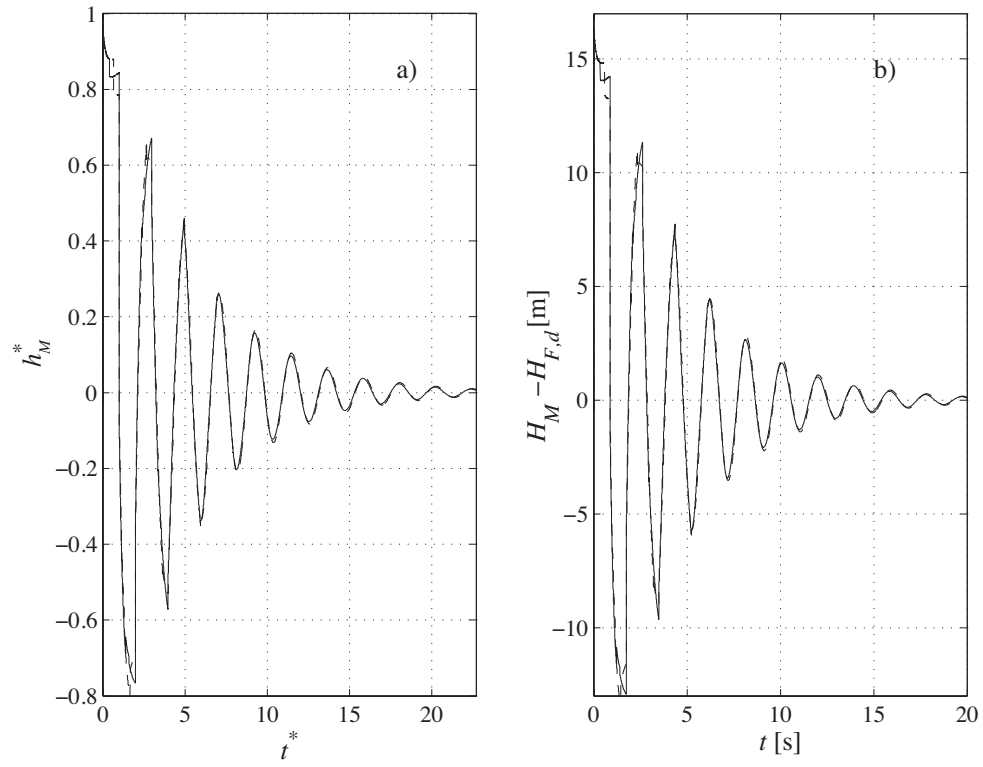


Figure 7.: Transients with the same value of the coefficients α and β of Eq. (7) for couple C (solid lines) and D (dashed lines) of Fig. 4: a) dimensionless pressure signals; b) dimensional pressure signals.

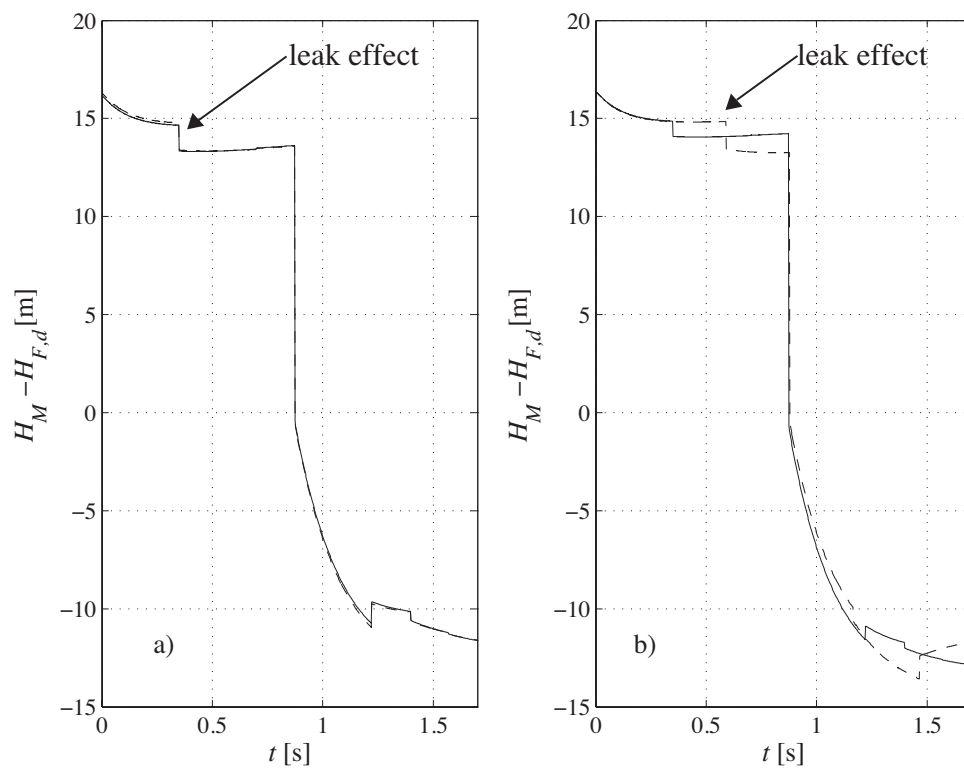


Figure 8.: Magnified vision of the first phase of transients of Figs. 6b and 7b with the effect of the leak in the pressure signal pointed out: a) couple A and B, b) couple C and D.

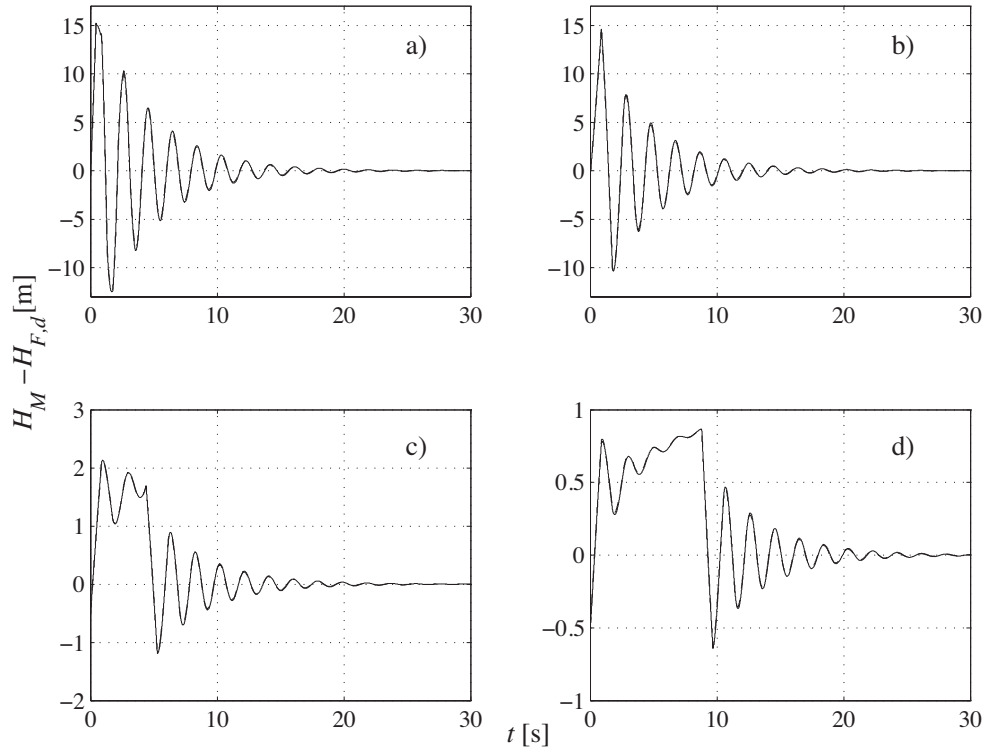


Figure 9.: Dimensional pressure signals for cases C and D of Fig. 4 for different values of the duration of the closing maneuver: a) $\Theta = 0.5$; b) $\Theta = 1$; c) $\Theta = 5$; d) $\Theta = 10$.

239 4. Conclusions and implications for real pipe systems

240 Because of the short duration of the tests and cheapness of the probes that are used
 241 (i.e., only pressure transducers), transient test-based techniques (TTBTs) apply for
 242 an important role within the management of pressurized pipe systems. This justifies
 243 the interest for an in-depth check of the most appropriate methods for the analysis of
 244 the results of the transient tests.

245 In this paper, the transient response of a single pipe with a leak (*leaky pipe*) has been
 246 examined with the specific aim of evaluating the mechanisms governing the total
 247 damping of the pressure peaks after the completion of the maneuver. In fact, since
 248 in a leaky pipe the total damping of the pressure peaks is much larger than the one
 249 in an integer pipe (i.e., the intrinsic damping), in principle such a feature could be
 250 assumed as a sort of marker of the leak in the pressure signals acquired during the
 251 transient tests (Wang et al. 2002; Nixon, Ghidaoui, and Kolyshkin 2006).

252 The results of the numerical tests, executed by means of a 1-D model calibrated on
 253 the basis of a huge series of laboratory tests and analyzed in the time-domain, show
 254 that the damping of the pressure peaks depends, as characteristic quantities, on the
 255 size and location of the leak and the initial pressure at the leak. On the contrary, the
 256 role of the initial mean velocity in the pipe downstream of the leak is quite negligible.
 257 Moreover, the numerical experiments confirm that an exponential law to simulate
 258 the behavior in time of the pressure peak damping can be assumed according to the
 259 case of the single integer pipe (Covas et al. 2004, 2005) and the pipe with a partially
 260 closed in-line valve (Meniconi et al. 2014).

261 The inspection of the charts where the coefficients of the exponential law are reported
262 as a function of the mentioned characteristic quantities highlights that the same
263 pressure peak damping may occur in pipe systems which differ in terms of leak size
264 and location. Moreover, it is shown that, for a given duration of the maneuver, if the
265 pressure peak damping is the same, negligible differences occur in the whole pressure
266 signal, with the exception of the first characteristic time.

267 In terms of the non-uniqueness of the correspondence between the total pressure peak
268 damping and the characteristics of the pipe system for a given transient, the obtained
269 results suggest that a more in-depth analysis of such a feature is needed. Therefore,
270 in future work different pipe materials (e.g., metallic) and more complex pipe systems
271 will be examined from both the numerical and experimental point of view.

272

273

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279 References

- 280 Babbitt, HE, FC Amsbary, and DR Gwinn. 1920. “The detection of leaks in underground
281 pipes.” *Journal of the American Water Works Association, AWWA* 7 (4): 589–595.
- 282 Brunone, B. 1999. “A transient test-based technique for leak detection in outfall pipes.” *J. of*
283 *Water Resources Planning and Management, ASCE* 125 (5): 302–306.
- 284 Brunone, B, M Ferrante, and S Meniconi. 2008. “Portable pressure wave-maker for leak detec-
285 tion and pipe system characterization.” *J. of American Water Works Association, AWWA*
286 100 (4): 108–116.
- 287 Brunone, B, S Meniconi, C Capponi, and M Ferrante. 2015. “Leak-induced pressure decay
288 during transients in viscoelastic pipes. Preliminary results.” *Procedia Engineering, Elsevier*
289 117: 243–252.
- 290 Capponi, C, M Ferrante, AC Zecchin, and J Gong. 2017. “Leak detection in a branched sys-
291 tem by inverse transient analysis with the admittance matrix method.” *Water Resources*
292 *Management* 31 (13): 4075–4089.
- 293 Colombo, AF, PJ Lee, and BW Karney. 2009. “A selective literature review of transient-based
294 leak detection methods.” *Journal of Hydro-Environment Research* 2 (4): 212–227.
- 295 Covas, D, and H Ramos. 2010. “Case studies of leak detection and location in water pipe
296 systems by inverse transient analysis.” *J. of Water Resources Planning and Management,*
297 *ASCE* 136 (2): 248–257.
- 298 Covas, D, I Stoianov, JF Mano, H Ramos, N Graham, and C Maksimovic. 2004. “The dynamic
299 effect of pipe-wall viscoelasticity in hydraulic transients. Part I - experimental analysis and
300 creep characterization.” *Journal of Hydraulic Research, IAHR* 42 (5): 517–532.
- 301 Covas, D, I Stoianov, JF Mano, H Ramos, N Graham, and C Maksimovic. 2005. “The dynamic
302 effect of pipe-wall viscoelasticity in hydraulic transients. Part II - model development, cali-
303 bration and verification.” *Journal of Hydraulic Research, IAHR* 43 (1): 56–70.
- 304 Duan, HF, MS Ghidaoui, PJ Lee, and YK Tung. 2010a. “Unsteady friction and visco-elasticity
305 in pipe fluid transients.” *Journal of Hydraulic Research, IAHR* 48 (3): 354–362.
- 306 Duan, HF, PJ Lee, MS Ghidaoui, and YK Tung. 2010b. “Essential system response information
307 for transient-based leak detection methods.” *Journal of Hydraulic Research* 48 (5): 650–657.
- 308 Duan, HF, PJ Lee, MS Ghidaoui, and YK Tung. 2011. “Extended blockage detection in
309 pipelines by using the system frequency response analysis.” *Journal of Water Resources*

- 310 *Planning and Management, ASCE* 138 (1): 55–62.
- 311 Duan, HF, PJ Lee, MS Ghidaoui, and YK Tung. 2012. “System response function–based leak
312 detection in viscoelastic pipelines.” *Journal of Hydraulic Engineering* 138 (2): 143–153.
- 313 Duan, HF, PJ Lee, A Kashima, J Lu, MS Ghidaoui, and YK Tung. 2013. “Extended blockage
314 detection in pipes using the system frequency response: Analytical analysis and experimental
315 verification.” *Journal of Hydraulic Engineering, ASCE* 139 (7): 763–771.
- 316 Ferrante, M, B Brunone, S Meniconi, BW Karney, and C Massari. 2014. “Leak size, detectabil-
317 ity and test conditions in pressurized pipe systems.” *Water Resources Management* 28 (13):
318 4583–4598.
- 319 Jönsson, L. 1970. “Leak detection in pipelines using hydraulic transients.” *WIT Transactions*
320 *on Ecology and the Environment* 7: 343–352.
- 321 Jönsson, L, and M Larson. 1992. “Leak detection through hydraulic transient analysis.” In
322 *Pipeline systems*, 273–286. Springer.
- 323 Joukowsky, N. 1900. “Über den hydraulischen Stoss in Wasserleitungsröhren” (“On the hy-
324 draulic hammer in water supply pipes”) (in German).” *Mémoires de l’Académie Impériale*
325 *des Sciences de St.-Pétersbourg* 9 (5): 1–71.
- 326 Lee, PJ, MF Lambert, AR Simpson, JP Vitkovsky, and J Liggett. 2006. “Experimental verifica-
327 tion of the frequency response method for pipeline leak detection.” *J. of Hydraulic Research,*
328 *IAHR* 44 (5): 693–707.
- 329 Lee, PJ, JP Vitkovsky, MF Lambert, AR Simpson, and JA Liggett. 2005. “Frequency domain
330 analysis for detecting pipeline leaks.” *J. of Hydraulic Engineering, ASCE* 131 (7): 596–604.
- 331 Lee, PJ, JP Vitkovský, MF Lambert, AR Simpson, and JA Liggett. 2008. “Discrete blockage
332 detection in pipelines using the frequency response diagram: numerical study.” *Journal of*
333 *Hydraulic Engineering, ASCE* 134 (5): 658–663.
- 334 Liggett, J, and L-C Chen. 1994. “Inverse transient analysis in pipe networks.” *Journal of*
335 *Hydraulic Engineering, ASCE* 120 (8): 934–955.
- 336 Liou, CP. 1998. “Pipeline leak detection by impulse response extraction.” *J. of Fluids Engi-*
337 *neering, ASME* 120: 833–838.
- 338 Liou, JCP, and J Tian. 1995. “Leak detection - Transient flow simulation approaches.” *J. of*
339 *Energy Resources Technology* 117: 243–248.
- 340 Louati, M, and MS Ghidaoui. 2017. “High-frequency acoustic wave properties in a water-filled
341 pipe. Part 1: dispersion and multi-path behaviour.” *Journal of Hydraulic Research, IAHR*
342 55 (5): 613–631.
- 343 Louati, M, S Meniconi, MS Ghidaoui, and B Brunone. 2017. “Experimental study of the eigen-
344 frequency shift mechanism in a blocked pipe system.” *Journal of Hydraulic Engineering,*
345 *ASCE* 143 (10): 04017044.
- 346 Meniconi, S, B Brunone, and M Ferrante. 2012. “Water hammer pressure waves at cross-section
347 changes in series in viscoelastic pipes.” *Journal of Fluids and Structures* 33: 44–58.
- 348 Meniconi, S, B Brunone, M Ferrante, and C Capponi. 2016. “Mechanism of interaction of
349 pressure waves at a discrete partial blockage.” *Journal of Fluids and Structures* 62: 33–45.
- 350 Meniconi, S, B Brunone, M Ferrante, C Capponi, CA Carrettini, C Chiesa, D Segalini, and
351 EA Lanfranchi. 2015. “Anomaly pre-localization in distribution-transmission mains. Prelim-
352 inary field tests in the Milan pipe system.” *J. of Hydroinformatics, IWA* 17 (3): 377–389.
- 353 Meniconi, S, B Brunone, M Ferrante, and C Massari. 2011a. “Potential of transient tests
354 to diagnose real supply pipe systems: what can be done with a single extemporary test.”
355 *Journal of Water Resources Planning and Management, ASCE* 137 (2): 238–241.
- 356 Meniconi, S, B Brunone, M Ferrante, and C Massari. 2011b. “Small amplitude sharp pressure
357 waves to diagnose pipe systems.” *Water Resources Management* 25 (1): 79–96.
- 358 Meniconi, S, B Brunone, M Ferrante, and C Massari. 2011c. “Transient tests for locating and
359 sizing illegal branches in pipe systems.” *Journal of Hydroinformatics, IWA* 13 (3): 334–345.
- 360 Meniconi, S, B Brunone, M Ferrante, and C Massari. 2012. “Transient hydrodynamics of in-
361 line valves in viscoelastic pressurized pipes: long-period analysis.” *Experiments in Fluids* 53
362 (1): 265–275.
- 363 Meniconi, S, B Brunone, M Ferrante, and C Massari. 2014. “Energy dissipation and pressure

364 decay during transients in viscoelastic pipes with an in-line valve.” *Journal of Fluids and*
365 *Structures* 45: 235–249.

366 Meniconi, S, B Brunone, and M Frisinghelli. 2018. “On the role of minor branches, energy
367 dissipation, and small defects in the transient response of transmission mains.” *Water* 10
368 (2): 187.

369 Meniconi, S, B Brunone, M Frisinghelli, E Mazzetti, M Larentis, and C Costisella. 2017. “Safe
370 transients for pipe survey in a real transmission main by means of a portable device: the
371 case study of the Trento (I) supply system.” *Procedia Engineering, Elsevier* 186: 228–235.

372 Mpesha, W, MH Chaudhry, and SL Gassman. 2002. “Leak detection in pipes by frequency
373 response method using a step excitation.” *Journal of Hydraulic Research, IAHR* 40 (1):
374 55–62.

375 Mpesha, W, SL Gassman, and MH Chaudhry. 2001. “Leak detection in pipes by frequency
376 response method.” *Journal of Hydraulic Engineering, ASCE* 127 (2): 134–147.

377 Nixon, W, and MS Ghidaoui. 2007. “Numerical sensitivity study of unsteady friction in simple
378 systems with external flow.” *Journal of Hydraulic Engineering, ASCE* 133 (7): 736–749.

379 Nixon, W, MS Ghidaoui, and AA Kolyshkin. 2006. “Range of validity of the transient damping
380 leakage detection method.” *Journal of Hydraulic Engineering, ASCE* 132 (9): 944–957.

381 Ramos, H, D Covas, A Borga, and D Loureiro. 2004. “Surge damping analysis in pipe systems:
382 modelling and experiments.” *Journal of Hydraulic Research, IAHR* 42 (4): 413–425.

383 Stephens, ML, MF Lambert, AR Simpson, and JP Vitkovsky. 2011. “Calibrating the water-
384 hammer response of a field pipe network by using a mechanical damping model.” *Journal*
385 *of Hydraulic Engineering, ASCE* 137 (10): 1225–1237.

386 Taghvaei, M, SBM Beck, and J Boxall. 2010. “Leak detection in pipes using induced water
387 hammer pulses.” *Int. J. of COMADEM* 13 (1): 19–25.

388 Wang, X-J, MF Lambert, and AR Simpson. 2005. “Detection and location of a partial blockage
389 in a pipeline using damping of fluid transients.” *Journal of Water Resources, Planning and*
390 *Management, ASCE* 131 (3): 244–249.

391 Wang, X-J, MF Lambert, AR Simpson, JA Liggett, and JP Vitkovsky. 2002. “Leak detection
392 in pipelines using the damping of fluid transients.” *J. of Hydraulic Engineering, ASCE* 128
393 (7): 697–711.

394 Wiggert, DC. 1968. “Unsteady flows in lines with distributed leakage.” *Journal of the Hy-*
395 *draulics Division, ASCE* 94 (1): 143–162.

396 Zhang, C, AC Zecchin, MF Lambert, J Gong, and AR Simpson. 2018. “Multi-stage parameter-
397 constraining inverse transient analysis for pipeline condition assessment.” *Journal of Hy-*
398 *droinformatics, IWA* 20 (2): 281–300.